

Hydrology Study for:



Dated:

August 9, 2023

Prepared By:

Land Development Professionals

130 Prominence Court

Dawsonville, Georgia 30534



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Narrative

The Site is located at Mountain Road, Cumming, Forsyth Georgia. It is an undeveloped 10.95 acre tract. The proposed land use is a laydown yard for the City of Cumming Utilities. The laydown yard will be graveled with a heavy duty paved access drive. The offsite drainage areas are minimal as shown on the pre and post development maps. There is one drainage basin containing offsite and on site run off.

Stormwater modeling was performed with the "HydroCAD" modeling software package, a variable tailwater hydrograph routing software that combines elements of TR-20 with fundamental reach routing and pond routing principles. A copy of the software inputs and outputs are included in **Appendix D**.

24-hour rainfall totals for the model were taken from the NOAA Precipitation Frequency Data Server for the site location

The overall Total Suspended Solid (TSS) load percentage for the project site was calculated using accepted spreadsheet tools developed by the Atlanta Regional Commission. This analysis provides credit based on better site design methods and on the effect of treatment trains. A copy of the TSS load rate calculation is provided in **Appendix B**.

Water Quality of 100% of the first 1.0 inch of rainfall is provided by the proposed storm water pond.

Vicinity Map

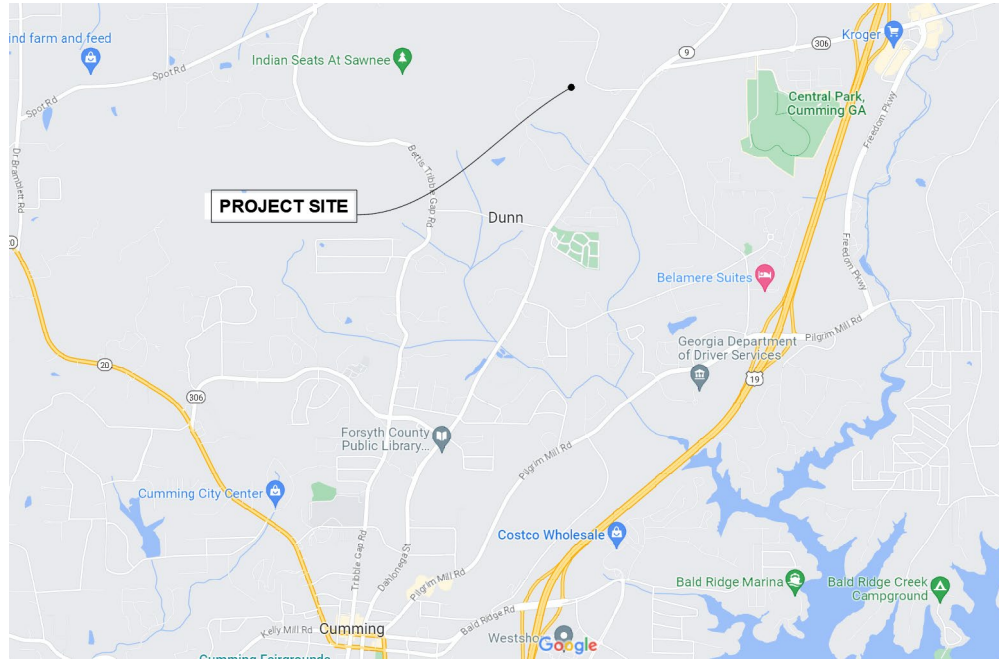


Figure 1 Vicinity Map

Discharge Summary

Discharge Summary

		1	2	5	10	25	50	100
Return Frequency (YR)								
Rainfall (in)		3.38	3.84	4.6	5.23	6.11	6.79	7.48
Study Point A	Predevelopment (cfs)	2.17	3.93	7.68	11.39	17.22	22.13	27.46
Study Point A	Post-Development Flow (cfs)	0.69	1.09	1.55	2.37	3.37	3.69	3.98

Proposed Project Scope:

The proposed project is a pipe laydown yard. The proposed infrastructure is a detention pond, OCS, and riprap apron.

BMPS's

In order to protect and enhance downstream environmental quality, temporary best management practices (BMPs) will be employed both during construction (temporary) and after construction is complete (permanent). Temporary BMPs will be identified and designed as part of the construction plan set's Erosion, Pollution, and Sedimentation Control Plan. A permanent Stormwater Pond is proposed for this project.

The Permanent provides all four methods of environmental protection as required by the Georgia Stormwater Management Manual; water quality treatment, downstream channel protection, overbank flood protection, and extreme flood protection.

Water Quality

Stormwater runoff from all the existing and proposed impervious surfaces in the project area will be directed to the detention pond, which provides water quality in a permanent wet pool of water equal to the water quality volume. Stormwater runoff displaces the water already present in the pool.

The stormwater detention pond provides a 80% reduction in TSS, meeting the regulatory threshold. Calculations for this TSS reduction were performed utilizing the Georgia Stormwater Management Manual Stormwater Quality Site Development Review Tool, version 2.2, and are appended for reference.

The water quality volume has been calculated as follows:

WATER QUALITY CALCULATIONS

WATER QUALITY STORAGE CALC

WQ	2480.0	CF
Rv	0.150	
Impervious surface percentage	0.112	
On-Impervious Drainage Area	0.569	24800 SF
Total onsite Drainage Area A	5.100	

WQ=
1.2"* impervious surface

Using the proposed pond storage volume, the permanent pool elevation (Water Quality Storage) will be 1158.50.

Channel Protection

To prevent an increase in downstream channel scour from the development, the runoff volume of the 1 year storm has been captured and drawn down over a period of 24 hours in the existing detention pond, in accordance with guidelines set by the GSMM. The channel protection volume for the 1-Year rainfall @ 3.38" is 0.219 AF. The required 6" channel protection orifice would be at 1260.9

Overbank Flood Protection and Extreme Flood Protection

A downstream hydrologic assessment for the overbank and extreme flood protection includes an unnamed tributary to Baldridge Creek. Overbank flood and Extreme Flood Protection are provided by the proposed stormwater management pond.

Stormwater Management Design Methodology

Stormwater modeling was performed with the "HydroCAD" modeling software package, a variable tailwater hydrograph routing software that combines elements of TR-20 with fundamental reach routing and pond routing principles. A copy of the software inputs and outputs are included in **Appendix D**.

24-hour rainfall totals for the model were taken from the NOAA Precipitation Frequency Data Server for the site location

The overall Total Suspended Solid (TSS) load percentage for the project site was calculated using accepted spreadsheet tools developed by the Atlanta Regional Commission. This analysis provides credit based on better site design methods and on the effect of treatment trains. A copy of the TSS load rate calculation is provided in **Appendix B**.

Water Quality Volume was calculated from the equation provided in the GSMM., and *Channel Protection* volume was calculated by methodologies within the GSMM. Copies of these spreadsheets for each pond are included in **Appendix B**.

Peak discharge calculations were performed using methodology from the Natural Resource Conservation Service's Technical Release 20, (NRCS TR-20) typically referred to as "the SCS

Method.” Watersheds were delineated on field run survey where available, and on USGS Quadrangle Maps where not available.

Stormwater modeling was performed with the “HydroCAD” modeling software package, a variable tailwater hydrograph routing software that combines elements of TR-20 with fundamental reach routing and pond routing principles. A copy of the software inputs and outputs are included in **Appendix D**.

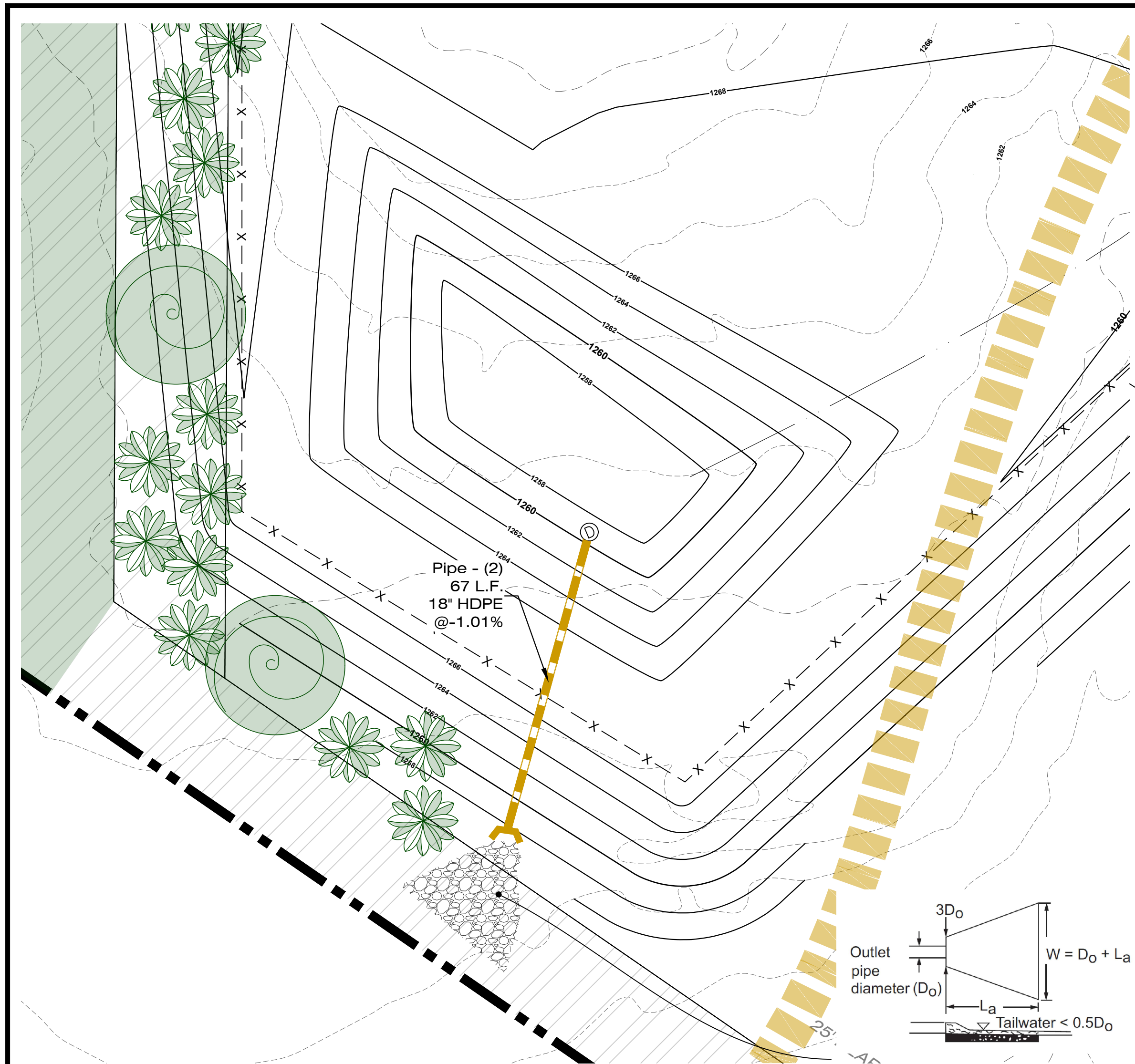
24-hour rainfall totals for the model were taken from the NOAA Precipitation Frequency Data Server for the site location.

Proposed Pond Stage Storage

Elevation (feet)	Cum.Store (acre-feet)
1,257.00	0.000
1,258.00	0.090
1,259.00	0.167
1,260.00	0.250
1,261.00	0.375
1,262.00	0.514
1,263.00	0.689
1,264.00	0.874
1,265.00	1.359
1,266.00	1.712
1,267.00	2.498

All stormwater management requirements have been satisfied with the existing wet detention pond. Water quality and Channel Protection are adequately designed to meet GSMM requirements.

Stormwater Detention Pond



4 FT SQ. PRECAST CONCRETE (INSIDE DIMENSIONS)
OUTLET CONTROL STRUCTURE WITH TOP AND COVER.

100' YEAR ELEVATION 1265.00
25' YEAR ELEVATION 1264.00

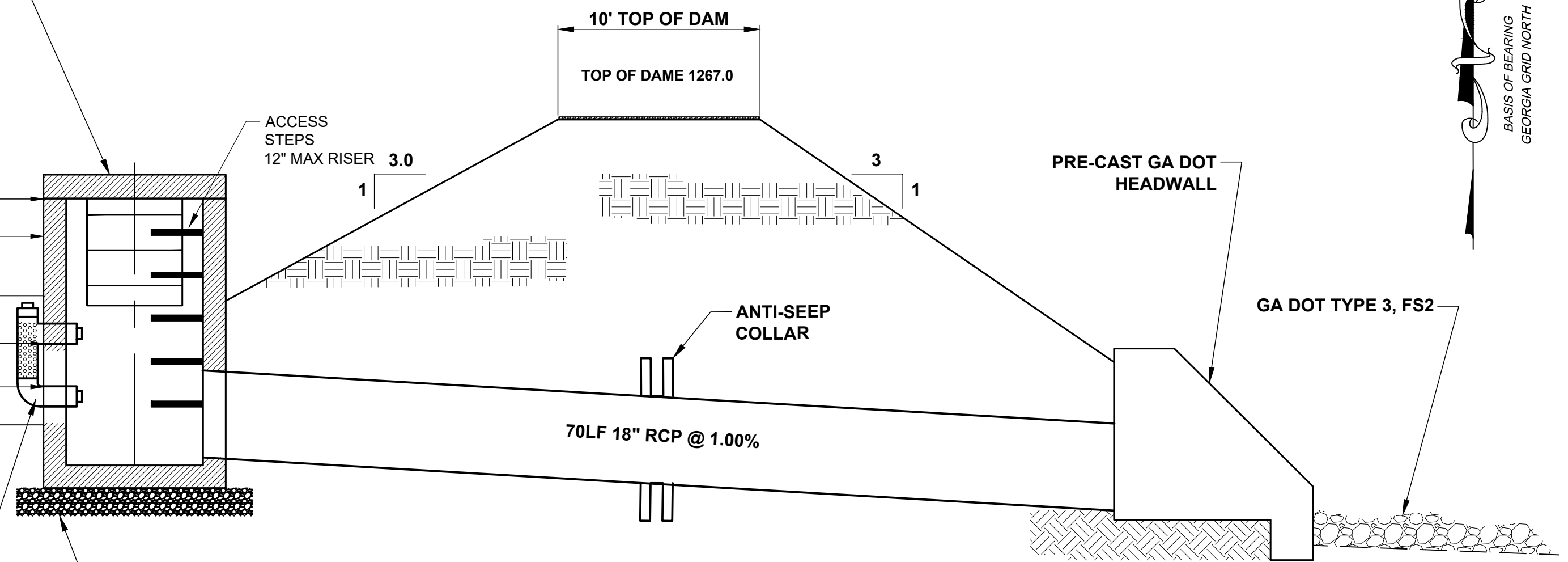
CHANNEL PROTECTION ELEVATION 1260.9'
WATER QUALITY ELEVATION 1258.50"

POND BOTTOM
EL. 1257

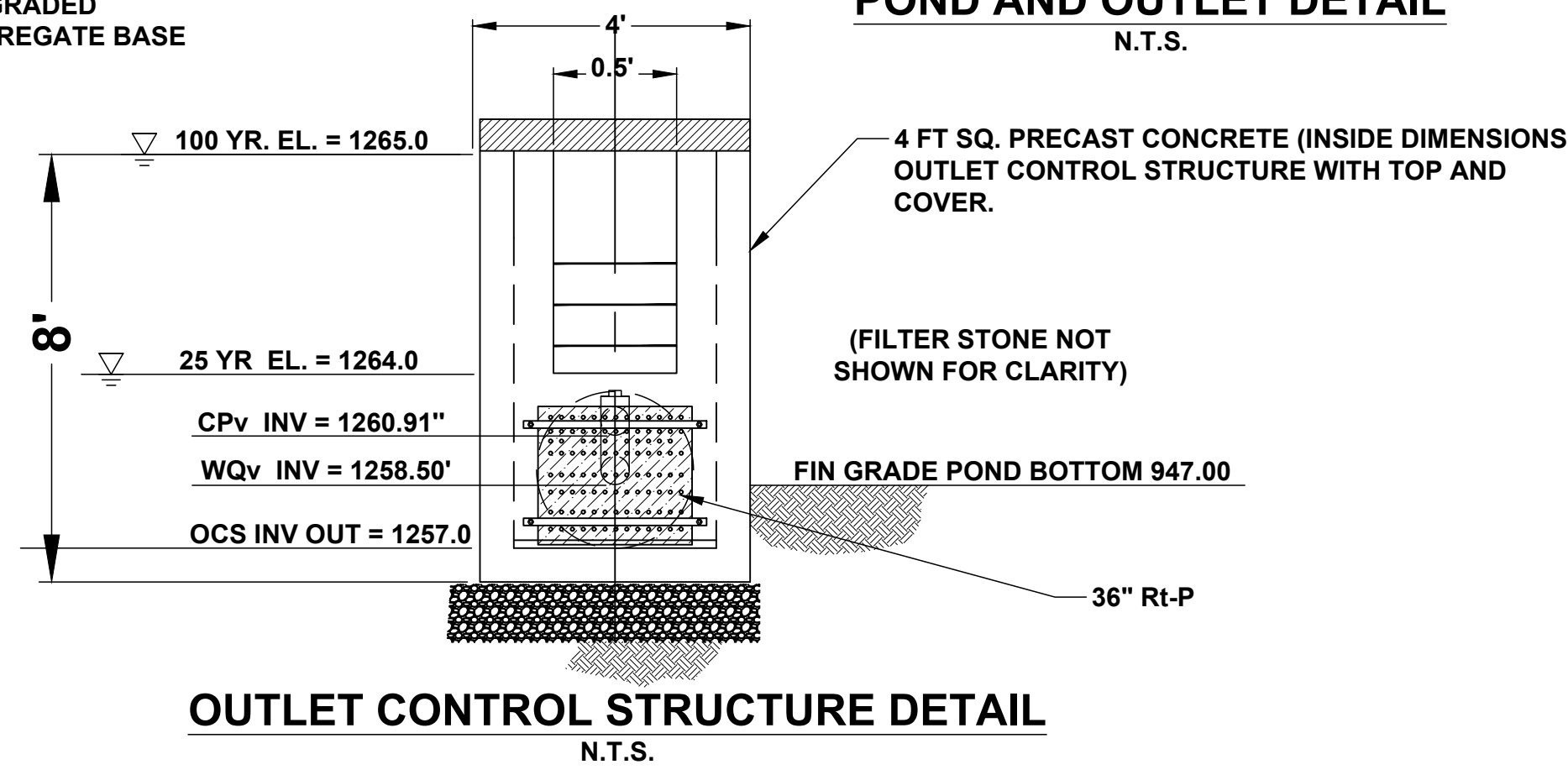
FILTER STONE WRAPPED
WITH NON-WOVEN
FILTER CLOTH

36" PERFORATED HALF-ROUND
CMP PERMANENTLY AFFIXED TO
STRUCTURE

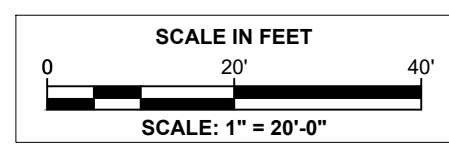
6" PVC SCH 80 PERFORATED PIPE
WITH ORIFICE DRILLED IN PLUG CAP
FOR WATER QUALITY AND CHANNEL
PROTECTION



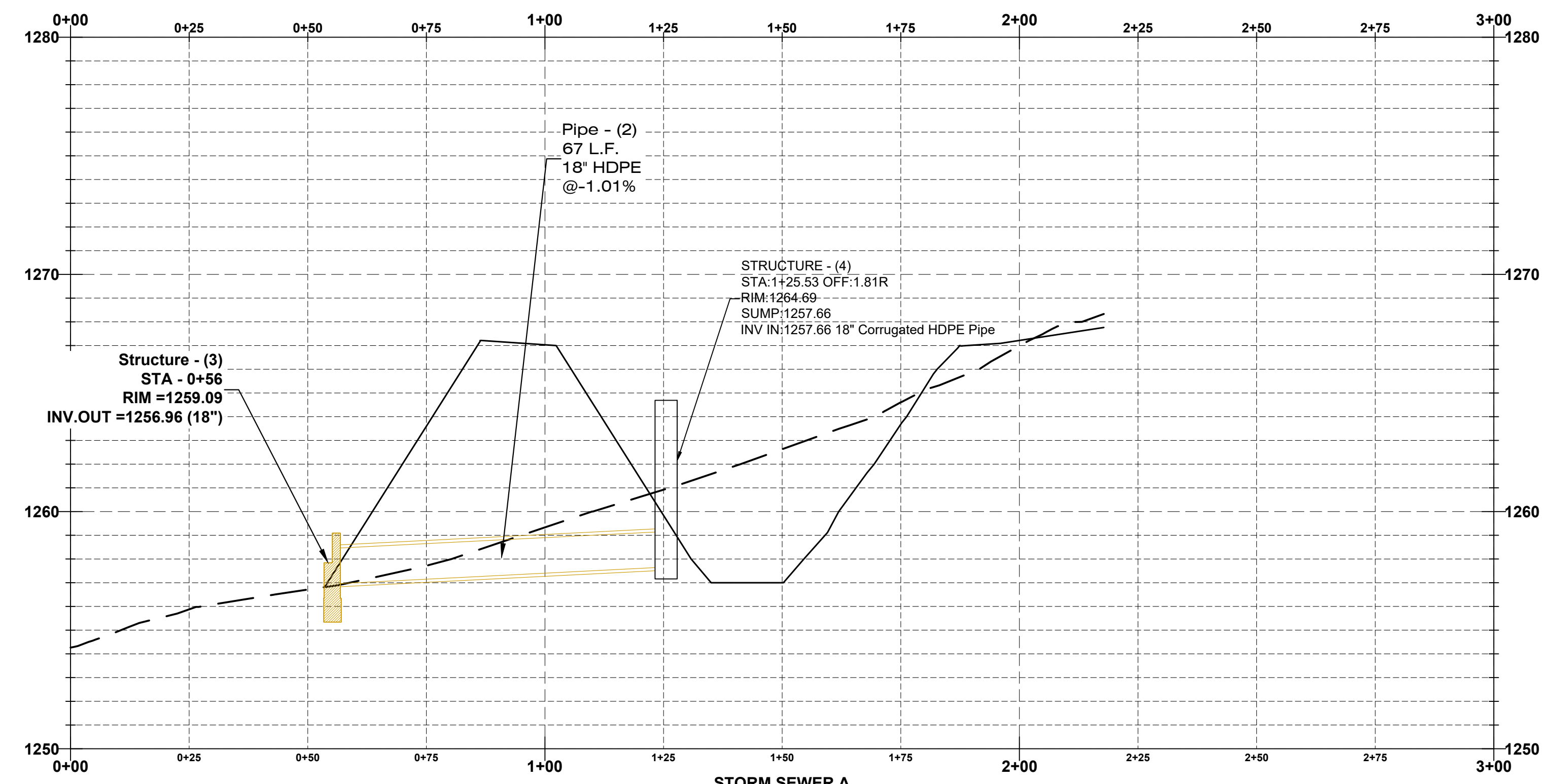
POND AND OUTLET DETAIL
N.T.S.



OUTLET CONTROL STRUCTURE DETAIL
N.T.S.



St	3Do	La	W	Do
	4.5'	11'	15.5'	.2'



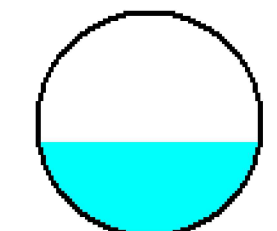
STORM SEWER A
STAT 0+00 - 3+00
SCALE: VERT 1"=8
HORIZ 1"=40

Inflow Area = 11.800 ac, 5.08% Impervious, Inflow Depth > 2.19" for 100-yr event
Inflow = 3.98 cfs @ 13.83 hrs, Volume= 2.156 af
Outflow = 3.98 cfs @ 13.82 hrs, Volume= 2.154 af, Atten= 0%, Lag= 0.0 min

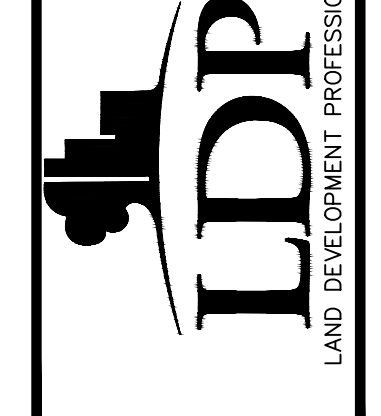
Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Max. Velocity= 5.87 fps, Min. Travel Time= 0.2 min
Avg. Velocity= 5.51 fps, Avg. Travel Time= 0.2 min

Peak Storage= 47 cf @ 13.82 hrs
Average Depth at Peak Storage= 0.61', Surface Width= 1.47'
Bank-Full Depth= 1.50' Flow Area= 1.8 sf, Capacity= 11.38 cfs

18.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 70.0' Slope= 0.0100 '
Inlet Invert= 1,257.66', Outlet Invert= 1,256.96'

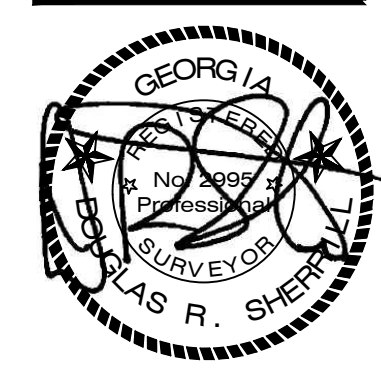


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131 Prominence Court
Dunwoody, GA 30344
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LSF001192



June 27, 2023
FIELD CREW: LDP
DRAWN BY: DRS
Job Number:
5151.001
DRWG FILE: 5151
FIELD DATE:
6/13/2023
Checked By: LDP

Description:
NOT FOR CONSTRUCTION

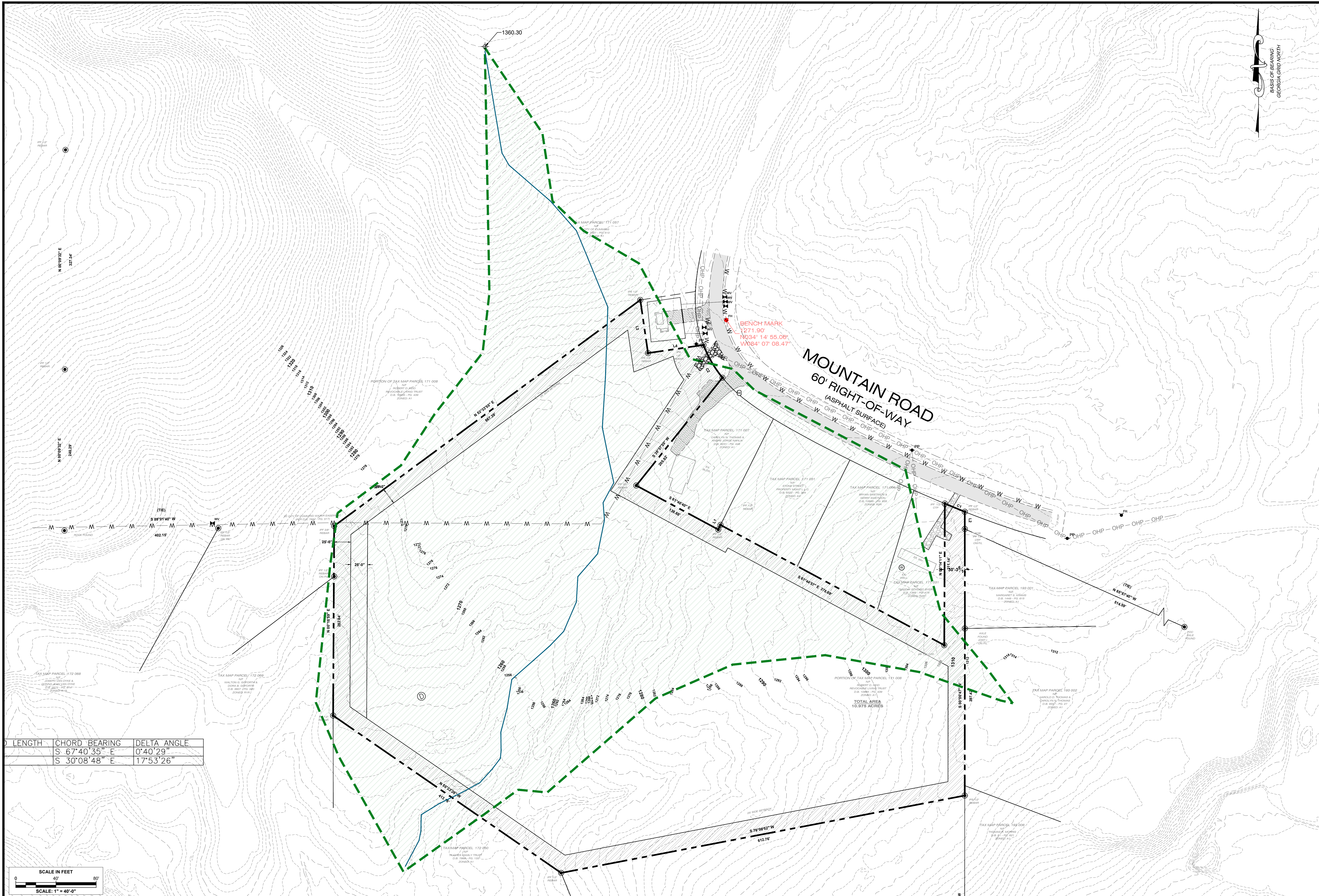


CITY OF CUMMING:
DETENTION POND DETAILS
Located In:
Land Lots 833 & 896 - 3rd District - 1st Section
Forsyth County, Georgia

Sheet No.
1-C-3

Drainage Basin Exhibits

Predevelopment Drainage Basin



LAND SURVEYING AND PLANNING
 BASIS OF BEARING
 GEORGIA GRID NORTH

LAND SURVEYING AND PLANNING
 GA #2995
 131 Prominence Court
 Office: 706.300.7178
 www.LDPofga.com
 doug@ldpofga.com
 LSP001192

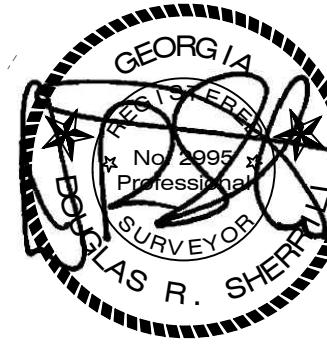
LDP
 LAND DEVELOPMENT PROFESSIONALS



June 27, 2023
 FIELD CREW: LDP
 DRAWN BY: DRS
 Job Number:
 5151.001
 DRWG FILE: 5151
 FIELD DATE:
 6/13/2023
 Checked By: LDP

Date:	Description:

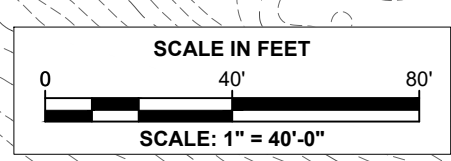
NOT FOR CONSTRUCTION



CITY OF CUMMING:
PREDEVELOPED DRAINAGE PLAN
 Located In:
 Land Lots 833 & 866 - 3rd District - 1st Station
 Forsyth County, Georgia

Sheet No.
1-C-2

LENGTH	CHORD BEARING	DELTA ANGLE
	S 67°40'35" E	0°40'29"
	S 30°08'48" E	17°53'26"



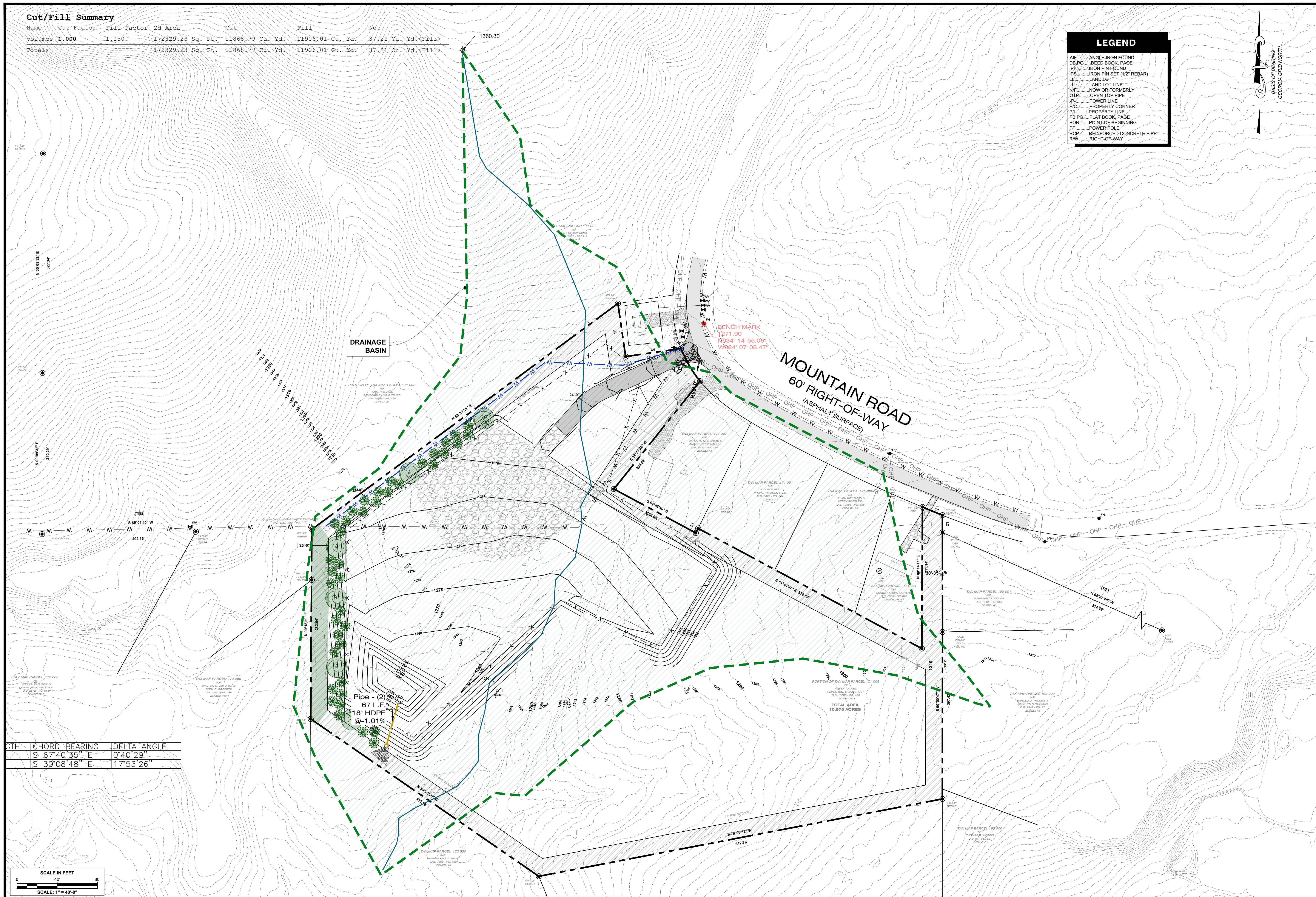
Post-Development Drainage Basin Exhibit

Cut/Fill Summary

Name	Cut Factor	Fill Factor	2d Area	Cut	Fill	Net
volumes	1.000	1.150	172329.23 Sq. Ft.	11868.79 Cu. Yd.	11906.01 Cu. Yd.	37.21 Cu. Yd.<Fill>
Totals			172329.23 Sq. Ft.	11868.79 Cu. Yd.	11906.01 Cu. Yd.	37.21 Cu. Yd.<Fill>

LEGEND

- AI.....ANGLE IRON FOUND
- DB,PG.....DEED BOOK, PAGE
- IP6.....IRON PIN FOUND
- LL.....LAND LOT
- LLL.....LAND LOT LINE
- NF.....NOW OR FORMERLY
- OT.....OPEN TOP PIPE
- P.....POWER LINE
- P/C.....PROPERTY CORNER
- PL.....PROPERTY LINE
- PL,PG.....PLAT BOOK, PAGE
- POB.....POINT OF BEGINNING
- PP.....POWER POLE
- RCP.....REINFORCED CONCRETE PIPE
- RW.....RIGHT-OF-WAY



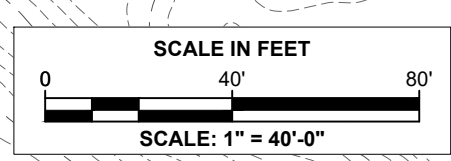
DRAINAGE BASIN

MOUNTAIN ROAD
60' RIGHT-OF-WAY
(ASPHALT SURFACE)

BENCHMARK
1271.90'
N034° 14' 55.08"
W084° 07' 08.47"

Pipe - (2)
67 L.F.
18" HDPE
@ -1.01%

GTH	CHORD BEARING	DELTA ANGLE
	S 67°40'35" E	0°40'29"
	S 30°08'48" E	17°53'26"



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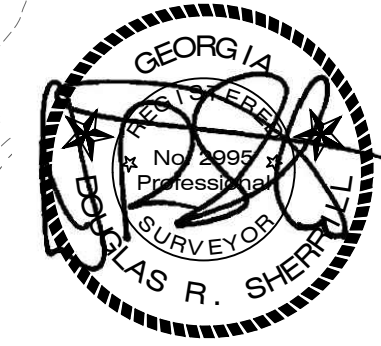
CUMMING UTILITIES

June 27, 2023
FIELD CREW: LDP
DRAWN BY: DRS
Job Number:
5151.001
DRWG FILE: 5151
FIELD DATE:
6/13/2023
Checked By: LDP

REVISIONS

Date:	Description:

NOT FOR CONSTRUCTION



CITY OF CUMMING:
POST DEVELOPED DRAINAGE BASIN
Located In:
Land Lots 833 & 866 - 3rd District - 1st Station
Forsyth County, Georgia

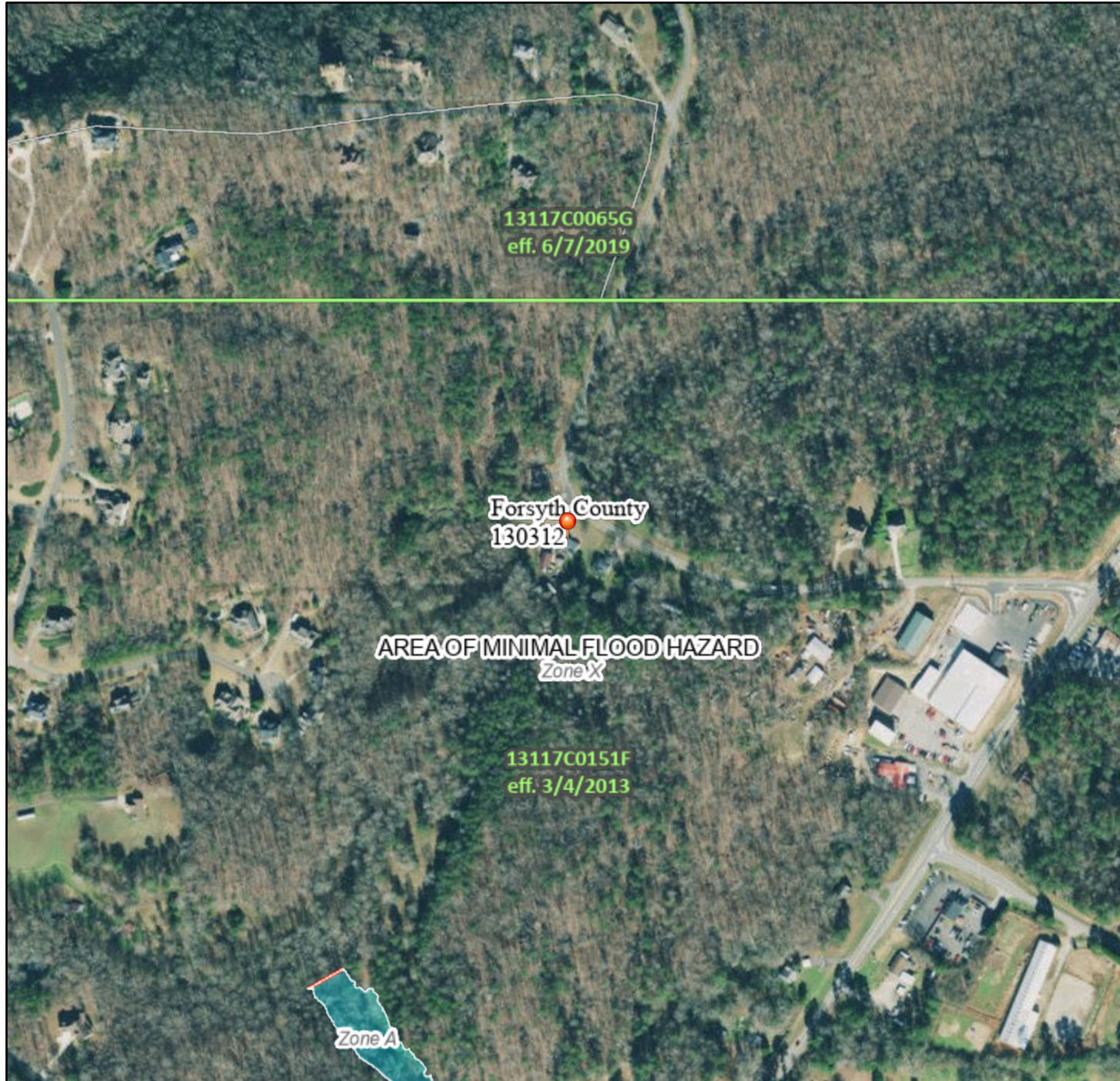
Sheet No.
1-C-2

Firmette

National Flood Hazard Layer FIRMette



84°7'27"W 34°15'8"N



Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

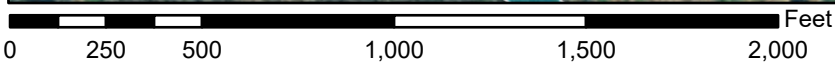
SPECIAL FLOOD HAZARD AREAS		Without Base Flood Elevation (BFE) <i>Zone A, V, A99</i>
		With BFE or Depth <i>Zone AE, AO, AH, VE, AR</i>
		Regulatory Floodway
OTHER AREAS OF FLOOD HAZARD		0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile <i>Zone X</i>
		Future Conditions 1% Annual Chance Flood Hazard <i>Zone X</i>
		Area with Reduced Flood Risk due to Levee. See Notes. <i>Zone X</i>
		Area with Flood Risk due to Levee <i>Zone D</i>
OTHER AREAS		NO SCREEN Area of Minimal Flood Hazard <i>Zone X</i>
		Effective LOMRs
GENERAL STRUCTURES		Area of Undetermined Flood Hazard <i>Zone D</i>
		Channel, Culvert, or Storm Sewer
		Levee, Dike, or Floodwall
OTHER FEATURES		20.2 Cross Sections with 1% Annual Chance
		17.5 Water Surface Elevation
		Coastal Transect
		Base Flood Elevation Line (BFE)
		Limit of Study
MAP PANELS		Jurisdiction Boundary
		Coastal Transect Baseline
		Profile Baseline
		Hydrographic Feature
		Digital Data Available
		No Digital Data Available
		Unmapped
		The pin displayed on the map is an approximate point selected by the user and does not represent an authoritative property location.



This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards

The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on **6/28/2023 at 8:01 AM** and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.



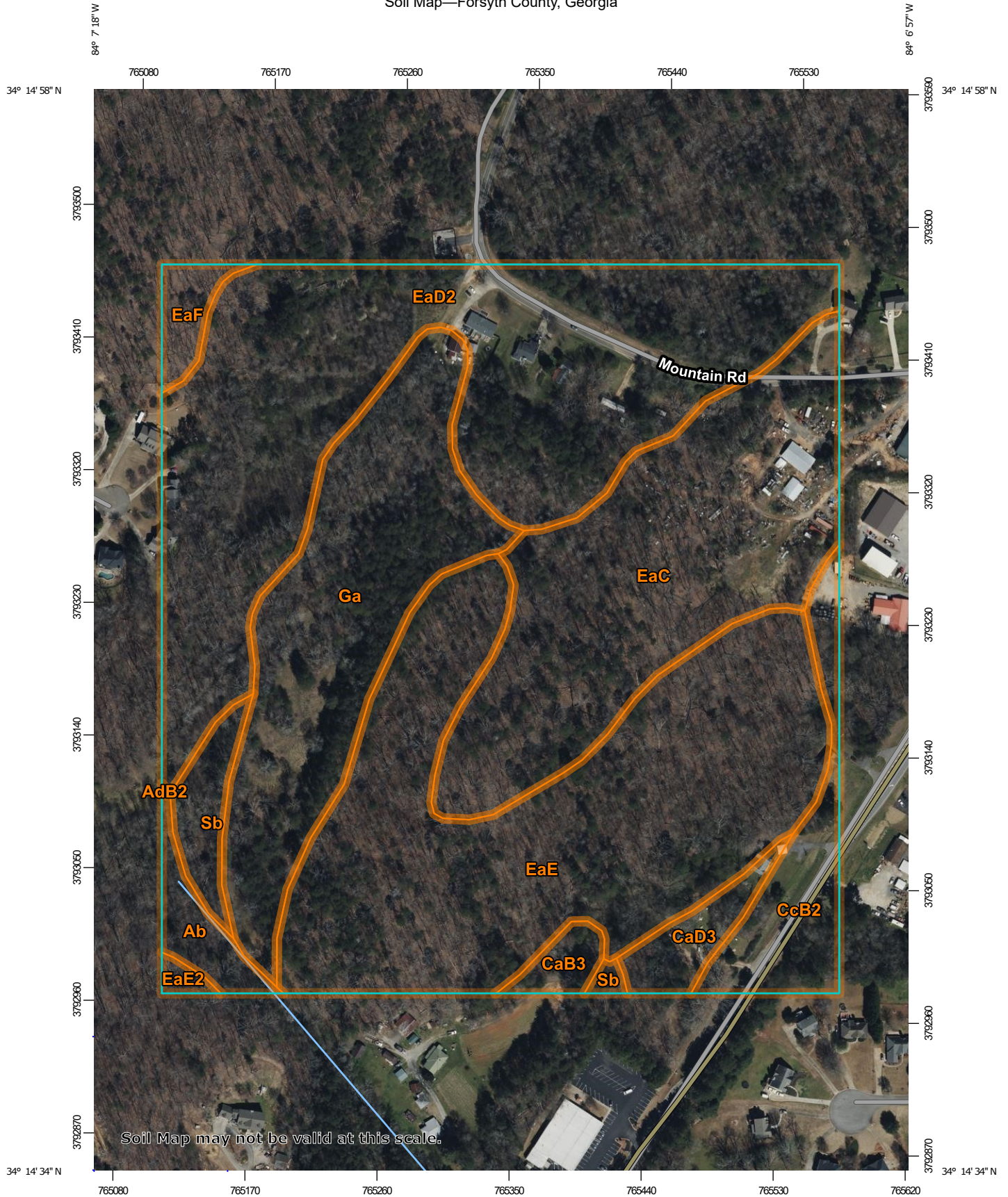
1:6,000

84°6'50"W 34°14'38"N

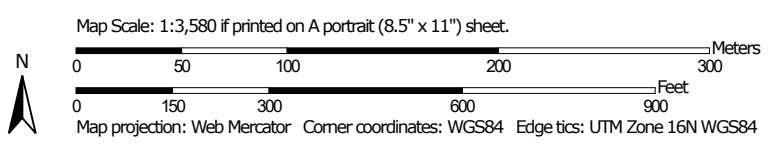
Basemap Imagery Source: USGS National Map 2023

Soils Information

Soil Map—Forsyth County, Georgia



Soil Map may not be valid at this scale.



MAP LEGEND

Area of Interest (AOI)

 Area of Interest (AOI)

Soils

 Soil Map Unit Polygons

 Soil Map Unit Lines

 Soil Map Unit Points

Special Point Features



Blowout



Borrow Pit



Clay Spot



Closed Depression



Gravel Pit



Gravelly Spot



Landfill



Lava Flow



Marsh or swamp



Mine or Quarry



Miscellaneous Water



Perennial Water



Rock Outcrop



Saline Spot



Sandy Spot



Severely Eroded Spot



Sinkhole



Slide or Slip



Sodic Spot



Spoil Area



Stony Spot



Very Stony Spot



Wet Spot



Other



Special Line Features

Water Features



Streams and Canals

Transportation



Rails



Interstate Highways



US Routes



Major Roads



Local Roads

Background



Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:20,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service

Web Soil Survey URL:

Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Forsyth County, Georgia

Survey Area Data: Version 15, Sep 13, 2022

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Mar 14, 2022—Jun 21, 2022

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
Ab	Toccoa and Chewacla soils, 0 to 2 percent slopes, occasionally flooded	0.9	1.6%
AdB2	Appling sandy clay loam, eroded very gently sloping phase	0.0	0.0%
CaB3	Cecil clay loam, severely eroded very gently sloping phase	0.5	0.9%
CaD3	Cecil clay loam, severely eroded sloping phase	1.0	1.7%
CcB2	Cecil sandy loam, 2 to 6 percent slopes, moderately eroded	2.4	4.3%
EaC	Edgemont stony sandy loam, gently sloping phase	10.4	18.4%
EaD2	Edgemont stony sandy loam, eroded sloping phase	16.1	28.4%
EaE	Edgemont stony sandy loam, moderately steep phase	15.0	26.6%
EaE2	Edgemont stony sandy loam, eroded moderately steep phase	0.2	0.3%
EaF	Edgemont stony sandy loam, steep phase	0.6	1.1%
Ga	Gullied land, acid materials	8.3	14.7%
Sb	Severely gullied land	1.1	2.0%
Totals for Area of Interest		56.6	100.0%

Site Development Tool

IDF Curve

5/20/2020

Precipitation Frequency Data Server



NOAA Atlas 14, Volume 9, Version 2
 Location name: Cumming, Georgia, USA*
 Latitude: 34.2042°, Longitude: -84.1396°
 Elevation: 1203.23 ft**
 * source: ESRI Maps
 ** source: USGS



POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Deborah Martin, Sandra Pavlovic, Ishani Roy, Michael St. Laurent, Carl Trypaniak, Dale Unruh, Michael Yekta, Geoffrey Bonnin

NOAA, National Weather Service, Silver Spring, Maryland

[PF tabular](#) | [PF graphical](#) | [Maps & aerials](#)

PF tabular

PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches) ¹										
Duration	Average recurrence interval (years)									
	1	2	5	10	25	50	100	200	500	1000
5-min	0.416 (0.333-0.520)	0.477 (0.382-0.596)	0.578 (0.462-0.724)	0.665 (0.529-0.835)	0.788 (0.615-1.01)	0.886 (0.680-1.14)	0.988 (0.739-1.29)	1.09 (0.794-1.45)	1.23 (0.873-1.66)	1.34 (0.833-1.82)
10-min	0.609 (0.468-0.761)	0.698 (0.559-0.873)	0.847 (0.677-1.06)	0.974 (0.775-1.22)	1.15 (0.900-1.48)	1.30 (0.995-1.67)	1.44 (1.08-1.89)	1.60 (1.18-2.12)	1.80 (1.28-2.43)	1.96 (1.37-2.67)
15-min	0.743 (0.585-0.926)	0.951 (0.681-1.06)	1.03 (0.825-1.29)	1.19 (0.945-1.49)	1.41 (1.10-1.80)	1.58 (1.21-2.04)	1.76 (1.32-2.30)	1.95 (1.42-2.58)	2.20 (1.58-2.96)	2.39 (1.67-3.25)
30-min	1.04 (0.834-1.30)	1.20 (0.957-1.50)	1.45 (1.18-1.82)	1.67 (1.33-2.10)	1.98 (1.54-2.53)	2.22 (1.70-2.86)	2.46 (1.85-3.22)	2.72 (1.98-3.61)	3.06 (2.17-4.13)	3.33 (2.32-4.52)
60-min	1.33 (1.07-1.67)	1.52 (1.22-1.90)	1.84 (1.47-2.30)	2.11 (1.68-2.65)	2.50 (1.98-3.21)	2.82 (2.16-3.64)	3.14 (2.38-4.11)	3.48 (2.54-4.62)	3.94 (2.80-5.32)	4.31 (3.00-5.85)
2-hr	1.63 (1.32-2.01)	1.85 (1.50-2.28)	2.23 (1.80-2.75)	2.56 (2.06-3.17)	3.03 (2.40-3.85)	3.42 (2.68-4.38)	3.82 (2.91-4.94)	4.24 (3.14-5.57)	4.82 (3.48-6.43)	5.29 (3.74-7.09)
3-hr	1.82 (1.49-2.23)	2.06 (1.68-2.52)	2.47 (2.01-3.03)	2.83 (2.29-3.48)	3.36 (2.68-4.23)	3.79 (2.97-4.80)	4.24 (3.26-5.45)	4.72 (3.53-6.16)	5.39 (3.93-7.14)	5.92 (4.22-7.88)
6-hr	2.24 (1.85-2.71)	2.52 (2.08-3.05)	3.01 (2.48-3.65)	3.44 (2.82-4.18)	4.06 (3.29-5.08)	4.58 (3.64-5.72)	5.11 (3.98-6.48)	5.68 (4.31-7.30)	6.47 (4.78-8.44)	7.09 (5.14-9.31)
12-hr	2.78 (2.33-3.32)	3.15 (2.63-3.76)	3.76 (3.14-4.49)	4.28 (3.56-5.13)	5.02 (4.11-6.15)	5.61 (4.52-6.91)	6.22 (4.90-7.76)	6.85 (5.27-8.68)	7.72 (5.78-9.92)	8.39 (6.17-10.9)
24-hr	3.38 (2.87-3.98)	3.84 (3.25-4.53)	4.60 (3.89-5.43)	5.23 (4.41-6.19)	6.11 (5.05-7.35)	6.79 (5.53-8.23)	7.48 (5.97-9.19)	8.19 (6.38-10.2)	9.13 (6.94-11.6)	9.85 (7.37-12.6)
2-day	3.97 (3.41-4.62)	4.51 (3.87-5.25)	5.40 (4.62-6.29)	6.14 (5.24-7.17)	7.17 (6.01-8.52)	7.98 (6.59-9.54)	8.79 (7.12-10.6)	9.62 (7.61-11.8)	10.7 (8.29-13.4)	11.6 (8.80-14.6)
3-day	4.37 (3.78-5.04)	4.93 (4.26-5.69)	5.87 (5.06-6.78)	6.66 (5.73-7.72)	7.79 (6.59-9.19)	8.68 (7.24-10.3)	9.59 (7.84-11.5)	10.5 (8.41-12.8)	11.8 (9.22-14.6)	12.8 (9.83-16.0)
4-day	4.72 (4.10-5.41)	5.28 (4.59-6.06)	6.25 (5.42-7.18)	7.08 (6.12-8.15)	8.27 (7.05-9.72)	9.23 (7.75-10.9)	10.2 (8.41-12.2)	11.2 (9.05-13.7)	12.7 (9.96-15.6)	13.8 (10.7-17.1)
7-day	5.81 (4.93-6.36)	6.25 (5.46-7.05)	7.25 (6.38-8.24)	8.17 (7.14-9.30)	9.50 (8.21-11.1)	10.6 (9.01-12.4)	11.7 (9.80-13.9)	12.8 (10.6-15.6)	14.5 (11.7-17.9)	16.0 (12.5-19.6)
10-day	6.37 (5.64-7.18)	7.03 (6.22-7.92)	8.16 (7.21-9.21)	9.16 (8.06-10.4)	10.6 (9.23-12.3)	11.8 (10.1-13.7)	13.1 (11.0-15.4)	14.4 (11.8-17.2)	16.2 (13.1-19.8)	17.7 (14.0-21.5)
20-day	8.52 (7.64-9.47)	9.39 (8.41-10.4)	10.8 (9.70-12.1)	12.1 (10.8-13.5)	13.9 (12.2-15.7)	15.3 (13.3-17.4)	16.7 (14.3-19.3)	18.2 (15.2-21.4)	20.3 (16.8-24.2)	21.9 (17.6-26.3)
30-day	10.5 (9.46-11.5)	11.5 (10.4-12.7)	13.2 (11.9-14.6)	14.6 (13.2-16.2)	16.6 (14.7-18.6)	18.1 (15.8-20.4)	19.6 (16.9-22.4)	21.1 (17.8-24.5)	23.2 (19.1-27.3)	24.7 (20.1-29.4)
45-day	13.1 (12.0-14.3)	14.4 (13.1-15.7)	16.4 (14.9-17.9)	17.9 (16.3-19.7)	20.0 (17.8-22.2)	21.6 (19.0-24.1)	23.1 (20.0-26.1)	24.5 (20.8-28.1)	26.4 (21.9-30.7)	27.7 (22.7-32.6)
60-day	15.6 (14.3-16.9)	17.0 (15.6-18.5)	19.2 (17.5-20.9)	20.9 (19.0-22.7)	23.0 (20.5-25.3)	24.5 (21.7-27.2)	25.9 (22.5-29.1)	27.2 (23.2-31.0)	28.8 (24.0-33.2)	29.8 (24.6-34.9)

¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS). Numbers in parentheses are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values. Please refer to NOAA Atlas 14 document for more information.

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PF graphical

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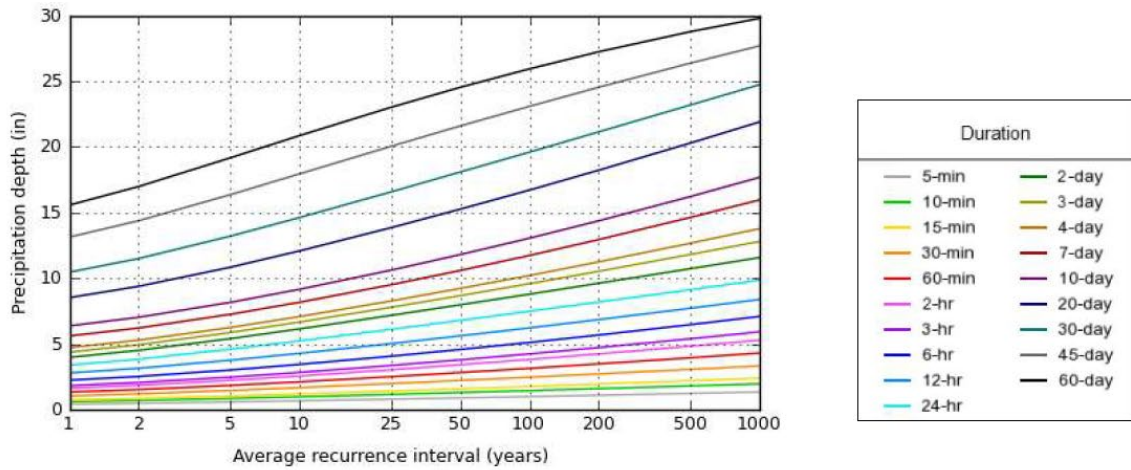
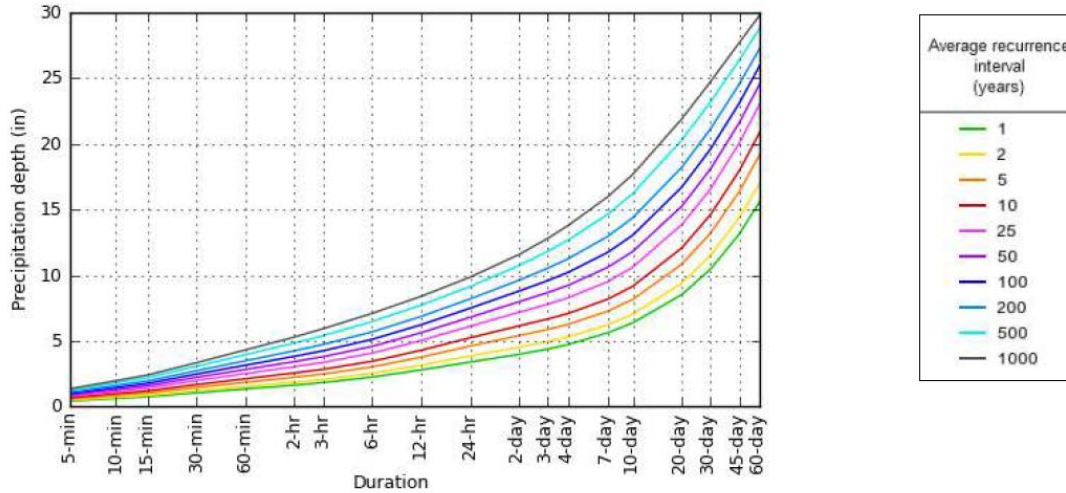
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5/20/2020

Precipitation Frequency Data Server

PDS-based depth-duration-frequency (DDF) curves
 Latitude: 34.2042°, Longitude: -84.1396°



NOAA Atlas 14, Volume 9, Version 2

Created (GMT): Wed May 20 21:42:21 2020

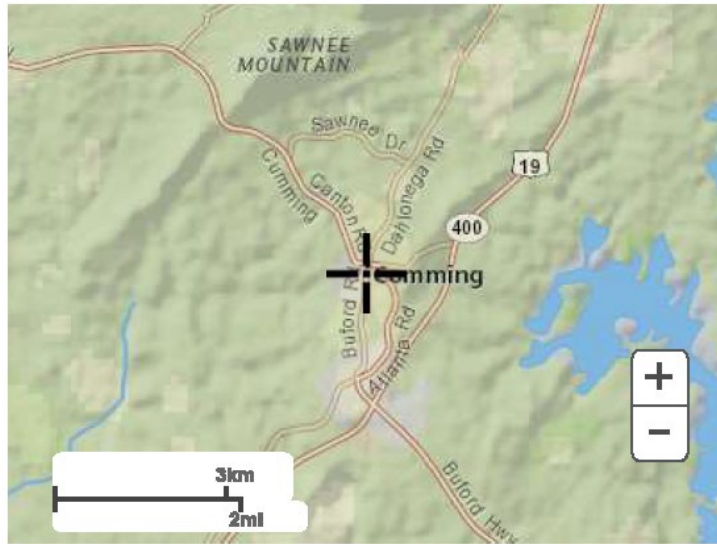
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Maps & aersials

Small scale terrain

5/20/2020

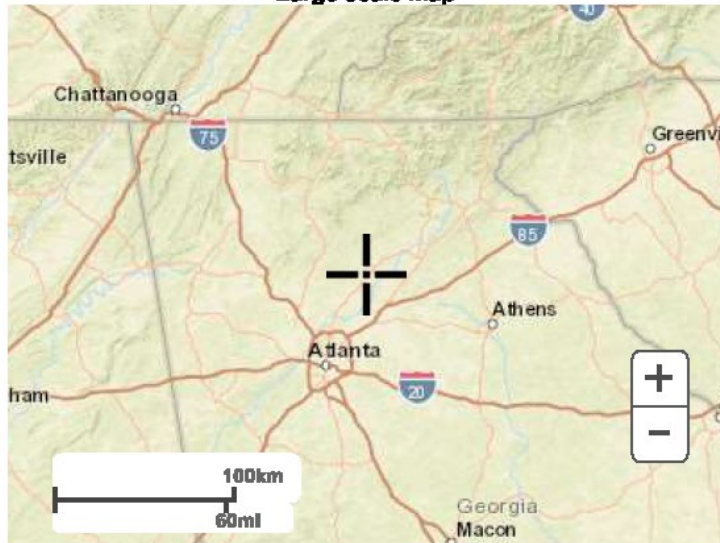
Precipitation Frequency Data Server



Large scale terrain



Large scale map



Large scale aerial

Precipitation Frequency Data Server



5/20/2020

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1325 East West Highway
Silver Spring, MD 20910
Questions?: HDSC.Questions@noaa.gov

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BMP Tracking Forms:

Stormwater Pond					
Maintenance Item	Condition				Comment
	Good	Marginal	Poor	N/A*	
General Inspection					
Access to the site is adequately maintained for inspection and maintenance.					
Area is clean (trash, debris, grass clippings, etc. removed).					
Inlet Structure					
Drainage ways (overland flow or pipes) to the practice are free of trash, debris, large branches, etc.					
Area around the inlet structure is mowed and grass clippings are removed.					
No evidence of gullies, rills, or excessive erosion around the inlet structure.					
Inlet pipe is in good condition, and water is going through the structure (i.e. no evidence of water going around the structure).					
Diversion structure (high flow bypass structure or other) is free of trash, debris, or sediment. Comment on overall condition of diversion structure and list type.					
Pretreatment (choose one)					
Forebay – area is free of trash, debris, and sediment.					
Filter Strip or Grass Channels – area is free of trash debris and sediment. Area has been mowed and grass clippings are removed. No evidence of erosion.					
Rock Lined Plunge Pools – area is free of trash debris and sediment. Rock thickness in pool is adequate.					
Main Treatment					
Main treatment area is free of trash, debris, and sediment.					
Erosion protection is present on site (i.e. turf reinforcement mats). Comment on types of erosion protection and evaluate condition.					
No algal growth along or within the pond.					
Native plants were used in the practice according to the planting plan. No undesirable vegetation.					
Practice seems to be working properly. No settling around the stormwater pond.					

Stormwater Pond					
Maintenance Item	Condition				Comment
	Good	Marginal	Poor	N/A*	
Comment on overall condition of stormwater pond.					
Vegetation within and around practice is maintained per landscaping plan. Grass clippings are removed.					
No significant sediment accumulation within the practice.					
No evidence of use of fertilizer on plants (fertilizer crusting on the surface of the soil, tips of leaves turning brown or yellow, blackened roots, etc.).					
Plants seem to be healthy and in good condition. Comment on condition of plants.					
Emergency Overflow					
Emergency overflow is free of trash, debris, and sediment.					
No evidence of erosion, scour, flooding, or animal activity around the structure.					
No evidence of erosion, scour, or flooding around the structure.					
Outlet Structure					
Outlet structure is free of trash, debris, and sediment.					
No evidence of erosion, scour, or flooding around the structure.					
Outlet structure does not appear to be blocked.					
No evidence of animal activity.					
No evidence of seepage on the downstream face.					
Results					
Overall condition of Stormwater Pond:					
Additional Comments					
Notes: * If a specific maintenance item was not checked, please check N/A and explain why in the appropriate comment box.					

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Project Notes

Rainfall events imported from "Detention Pond Assessment 2021.hcp"

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Rainfall Events Listing

Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
1	1-yr	Type II 24-hr		Default	24.00	1	3.38	2
2	2-yr	Type II 24-hr		Default	24.00	1	3.84	2
3	5-yr	Type II 24-hr		Default	24.00	1	4.60	2
4	10-yr	Type II 24-hr		Default	24.00	1	5.23	2
5	25-yr	Type II 24-hr		Default	24.00	1	6.11	2
6	50-yr	Type II 24-hr		Default	24.00	1	6.79	2
7	100-yr	Type II 24-hr		Default	24.00	1	7.48	2

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Type II 24-hr 1-yr Rainfall=3.38"

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Summary for Subcatchment 1S: PRE-DEVELOPMENT

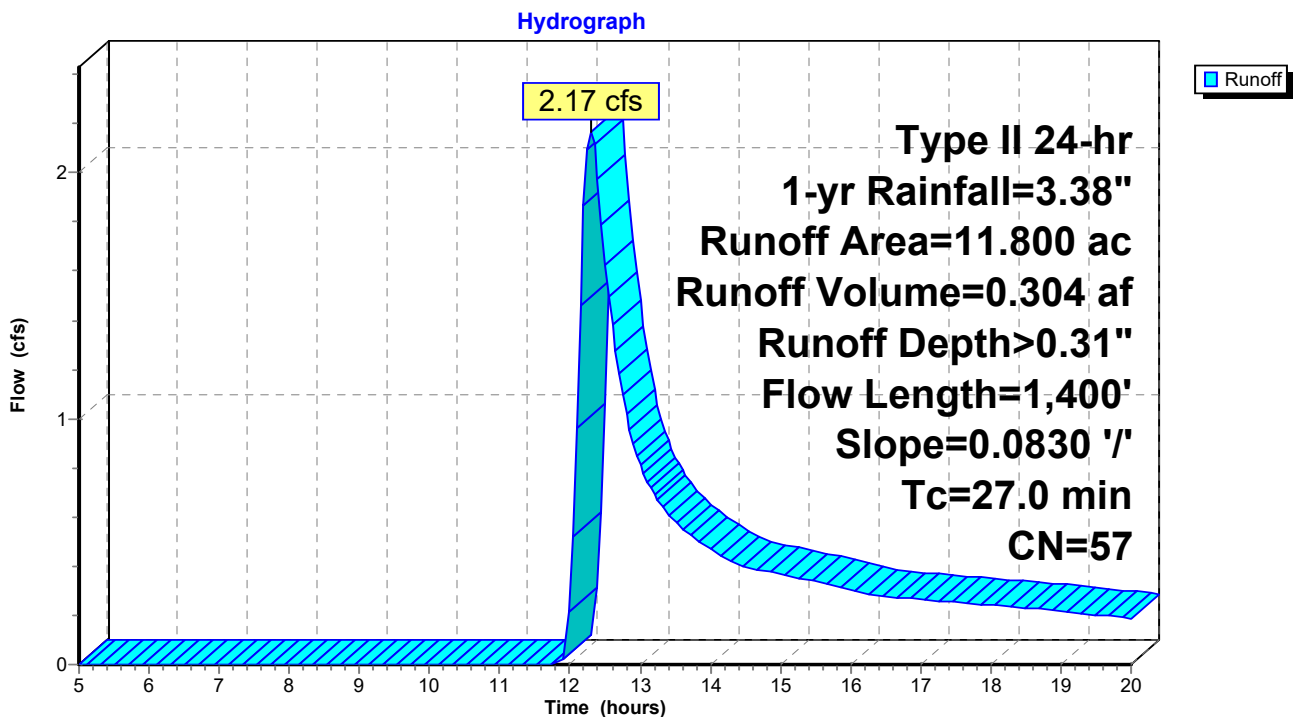
Runoff = 2.17 cfs @ 12.30 hrs, Volume= 0.304 af, Depth> 0.31"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
 Type II 24-hr 1-yr Rainfall=3.38"

Area (ac)	CN	Description
11.200	55	Woods, Good, HSG B
0.600	89	Paved roads w/open ditches, 50% imp, HSG B
11.800	57	Weighted Average
11.500		97.46% Pervious Area
0.300		2.54% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
27.0	1,400	0.0830	0.87		Lag/CN Method, TC

Subcatchment 1S: PRE-DEVELOPMENT



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Type II 24-hr 2-yr Rainfall=3.84"

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Summary for Subcatchment 1S: PRE-DEVELOPMENT

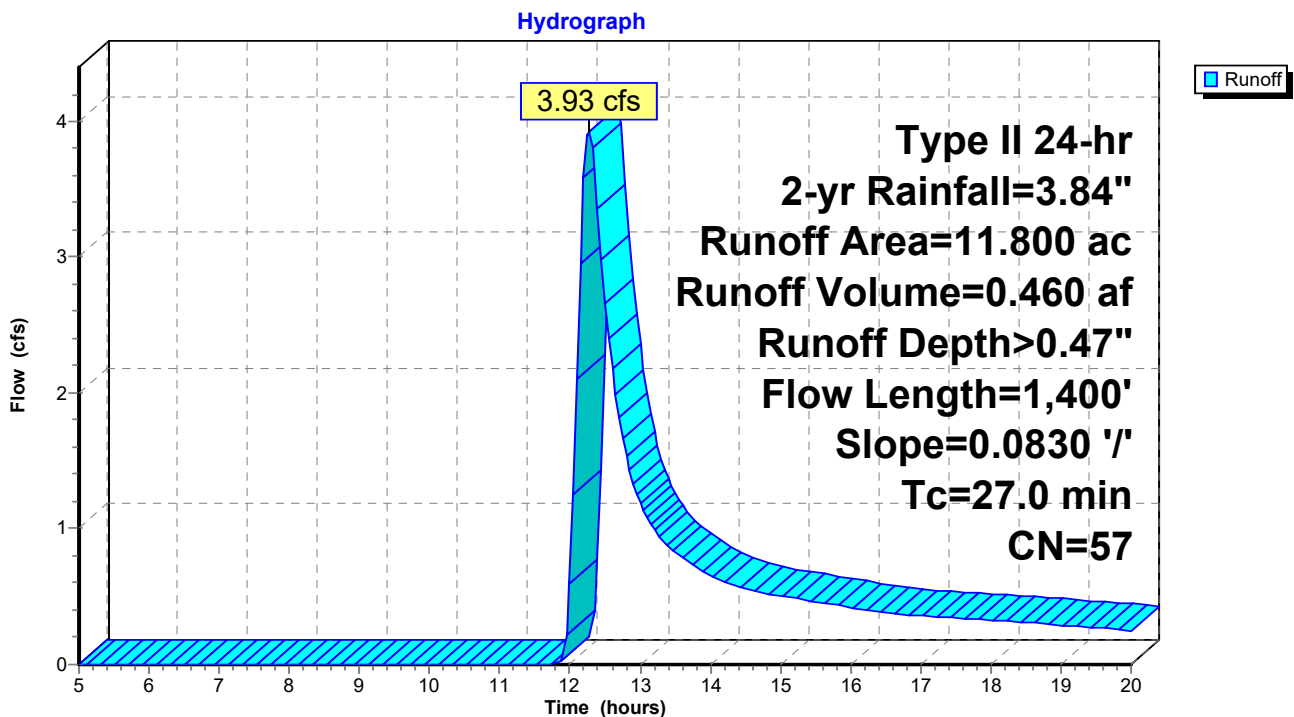
Runoff = 3.93 cfs @ 12.28 hrs, Volume= 0.460 af, Depth> 0.47"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
 Type II 24-hr 2-yr Rainfall=3.84"

Area (ac)	CN	Description
11.200	55	Woods, Good, HSG B
0.600	89	Paved roads w/open ditches, 50% imp, HSG B
11.800	57	Weighted Average
11.500		97.46% Pervious Area
0.300		2.54% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
27.0	1,400	0.0830	0.87		Lag/CN Method, TC

Subcatchment 1S: PRE-DEVELOPMENT



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Type II 24-hr 5-yr Rainfall=4.60"

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Summary for Subcatchment 1S: PRE-DEVELOPMENT

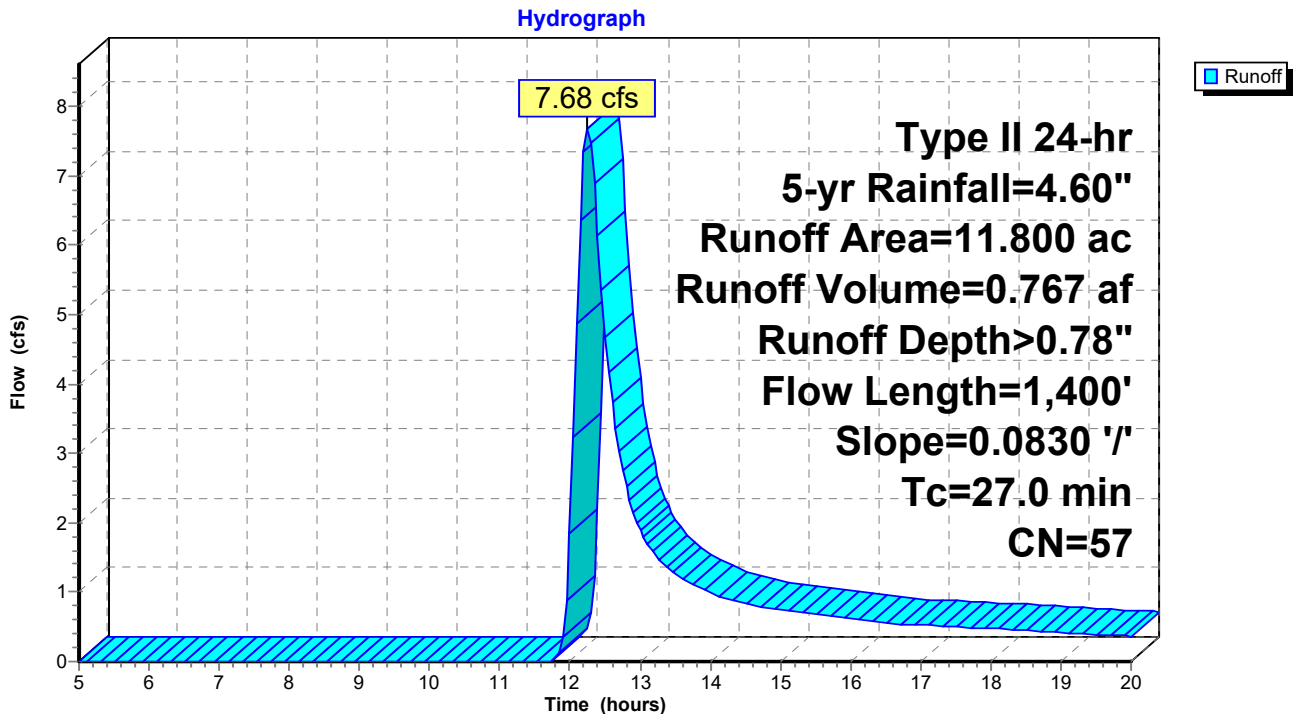
Runoff = 7.68 cfs @ 12.26 hrs, Volume= 0.767 af, Depth> 0.78"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
 Type II 24-hr 5-yr Rainfall=4.60"

Area (ac)	CN	Description
11.200	55	Woods, Good, HSG B
0.600	89	Paved roads w/open ditches, 50% imp, HSG B
11.800	57	Weighted Average
11.500		97.46% Pervious Area
0.300		2.54% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
27.0	1,400	0.0830	0.87		Lag/CN Method, TC

Subcatchment 1S: PRE-DEVELOPMENT



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Type II 24-hr 10-yr Rainfall=5.23"

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Summary for Subcatchment 1S: PRE-DEVELOPMENT

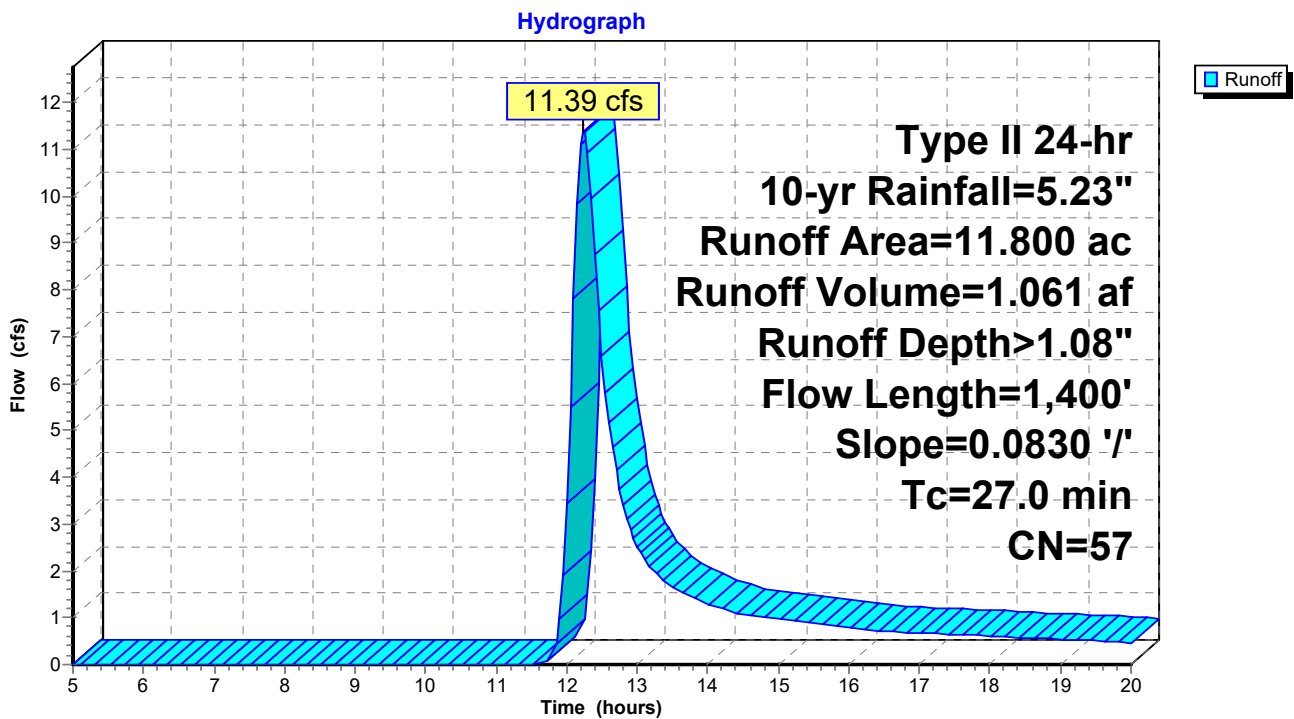
Runoff = 11.39 cfs @ 12.24 hrs, Volume= 1.061 af, Depth> 1.08"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
 Type II 24-hr 10-yr Rainfall=5.23"

Area (ac)	CN	Description
11.200	55	Woods, Good, HSG B
0.600	89	Paved roads w/open ditches, 50% imp, HSG B
11.800	57	Weighted Average
11.500		97.46% Pervious Area
0.300		2.54% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
27.0	1,400	0.0830	0.87		Lag/CN Method, TC

Subcatchment 1S: PRE-DEVELOPMENT



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Type II 24-hr 25-yr Rainfall=6.11"

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Summary for Subcatchment 1S: PRE-DEVELOPMENT

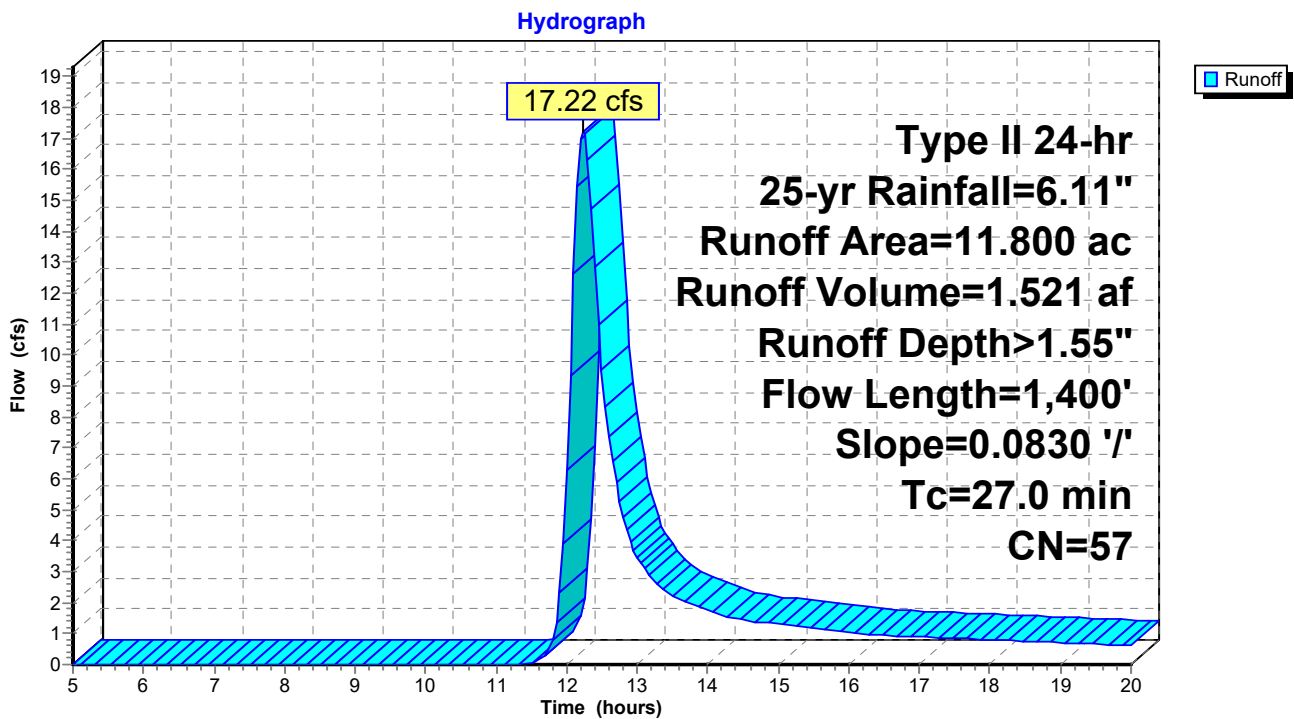
Runoff = 17.22 cfs @ 12.23 hrs, Volume= 1.521 af, Depth> 1.55"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 25-yr Rainfall=6.11"

Area (ac)	CN	Description
11.200	55	Woods, Good, HSG B
0.600	89	Paved roads w/open ditches, 50% imp, HSG B
11.800	57	Weighted Average
11.500		97.46% Pervious Area
0.300		2.54% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
27.0	1,400	0.0830	0.87		Lag/CN Method, TC

Subcatchment 1S: PRE-DEVELOPMENT



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Type II 24-hr 50-yr Rainfall=6.79"

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Summary for Subcatchment 1S: PRE-DEVELOPMENT

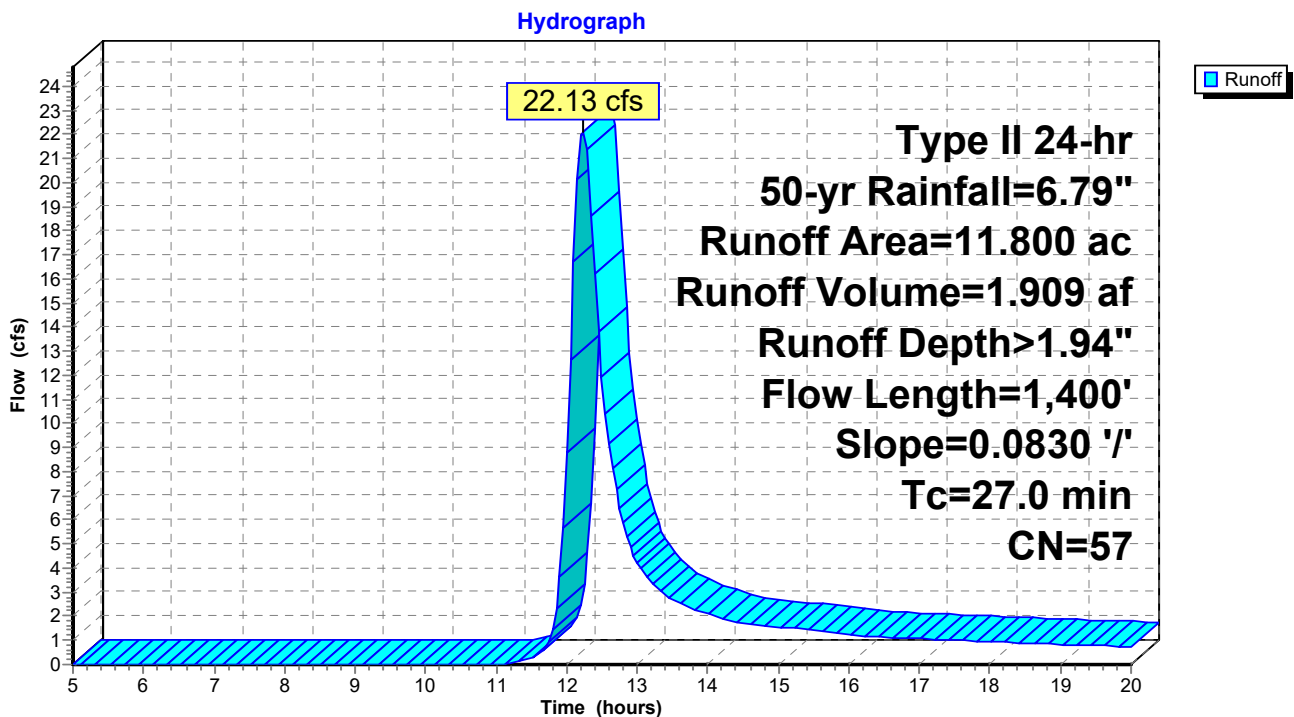
Runoff = 22.13 cfs @ 12.23 hrs, Volume= 1.909 af, Depth> 1.94"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 50-yr Rainfall=6.79"

Area (ac)	CN	Description
11.200	55	Woods, Good, HSG B
0.600	89	Paved roads w/open ditches, 50% imp, HSG B
11.800	57	Weighted Average
11.500		97.46% Pervious Area
0.300		2.54% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
27.0	1,400	0.0830	0.87		Lag/CN Method, TC

Subcatchment 1S: PRE-DEVELOPMENT



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Type II 24-hr 100-yr Rainfall=7.48"

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Summary for Subcatchment 1S: PRE-DEVELOPMENT

Runoff = 27.46 cfs @ 12.22 hrs, Volume= 2.328 af, Depth> 2.37"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-yr Rainfall=7.48"

Area (ac)	CN	Description
11.200	55	Woods, Good, HSG B
0.600	89	Paved roads w/open ditches, 50% imp, HSG B
11.800	57	Weighted Average
11.500		97.46% Pervious Area
0.300		2.54% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
27.0	1,400	0.0830	0.87		Lag/CN Method, TC

Subcatchment 1S: PRE-DEVELOPMENT

